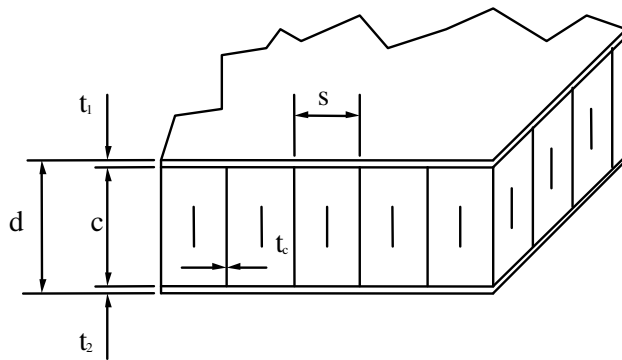


**Composite Engineer's Viewpoint**  
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**Designing with Composite Materials**  
**Part 7E – Detail Design – Sandwich Structures**

In this article we investigate the general design guidelines for sandwich structures with composite face sheets. Firstly, we define a sandwich structure as a face sheet (skin) of fibre reinforced composite material and a core of either honeycomb, or foam or some low density material. The dimensions of the skins and core are defined in the following diagram.



Where:

- $d$  = panel depth
- $t$  = facing thickness
- $c$  = core thickness
- $s$  = cell size (honeycomb)
- $t_c$  = honeycomb cell wall thickness

Note:

- $b$  = panel or beam width
- $h$  = distance between skin centroid

$$= d - \frac{(t_1 + t_2)}{2}$$

The benefits and disadvantages with foam or honeycomb core sandwich structure are as follows:

- Benefits:
  - High flexural stiffness-to-weight ratio
  - Energy absorption capability with crushing
  - Low heat transfer with low conductivity through-the-thickness
  - Noise and vibration insulation and reduction
  - Better bending strength-to-weight ratio efficiency (skins take the axial loads more efficiently)
- Disadvantages
  - Relativity low damage tolerance
  - Moisture absorption potential
  - Repairability
  - Edgewise crushing

The basic sandwich structure sizing requirements can be estimated from a simplified analysis approach. This approach assumes that the skin thickness at least 1/10<sup>th</sup> the thickness of the core.

Face sheets bending stresses:

$$\sigma_{skin} \approx \frac{M}{t_s hb}$$

where:  $M$  = Panel bending moment

$t_s$  = skin thickness

Skin dimpling stress:

$$\sigma_{crdimpling} \approx \frac{2E_s}{1-\nu_x\nu_y} \left(\frac{t_s}{s}\right)^2$$

Where:  $E_s$  = Composite skin Young's modulus  
 $\nu$  = Skin major and minor Poisson's ratios

Core shear stress:

Assumes that the core modulus  $E'_c \approx 0$

$$\tau_{core} \approx \frac{V}{hb}$$

where:  $V$  = section transverse shear load

Skin wrinkling stress:

$$\sigma_{crwrinkling} \approx 0.82E_s \sqrt{\frac{E_c t_s}{E_s t_c}}$$

Where:  $E_c$  = core compression modulus

Transverse Deflections:  $\delta_{total} = \delta_{bending} + \delta_{shear}$

Bending: 
$$\delta_{bending} = K_b \frac{Pl^3}{D}$$

Core Young's modulus:  $E'_c \approx 0$

Panel flexural rigidity: 
$$D \approx \frac{E_s t_s h^2}{2(1-\nu_x\nu_y)}$$

$K_b$  = see table below

Shear: 
$$\delta_{shear} = K_s \frac{Pl}{hG_c}$$

$G_c$  = core shear modulus

$K_s$  = see table below

Beam Type	$V_{max}$	$M_{max}$	$K_b$	$K_s$
Simply supported with uniformly distributed load	$0.5P$	$0.125Pl$	0.0130	0.125
Fixed with uniformly distributed load	$0.5P$	$0.0.833Pl$	0.0026	0.125
Simply supported with central point load	$0.5P$	$0.25Pl$	0.0208	0.25
Fixed supported with central point load	$0.5P$	$0.125Pl$	0.00521	0.25
Cantilever with uniformly distributed load	$P$	$0.5Pl$	0.125	0.5
Cantilever with end point load	$P$	$Pl$	0.0667	0.333

In the next article I will comment on the issue of interlaminar stresses in a little more detail. The basic areas of concern were interlaminar stresses are considered a potential problem and methods of reducing excessive interlaminar stress build-up. As always I welcome questions, comments and your point of view. Feel free to contact me via [r.heslehurst@adfa.edu.au](mailto:r.heslehurst@adfa.edu.au). I may publish your questions and comments, and my response in future articles.